



CTC 450 Review

- Friction Loss
 - Over a pipe length
 - Darcy-Weisbach (Moody's diagram)
 - Connections/fittings, etc.



Objectives

- Know how to set up a spreadsheet to solve a simple water distribution system using the Hardy-Cross method



Pipe Systems

- Water municipality systems consist of many junctions or nodes; many sources, and many outlets (loads)
- Object for designing a system is to deliver flow at some design pressure for the lowest cost
- Software makes the design of these systems easier than in the past; however, it's important to understand what the software is doing



Two parallel pipes

- If a pipe splits into two pipes how much flow will go into each pipe?
- Each pipe has a length, friction factor and diameter
- Head loss going through each pipe has to be equal



Two parallel pipes

$$f_1 * (L_1 / D_1) * (V_1^2 / 2g) = f_2 * (L_2 / D_2) * (V_2^2 / 2g)$$

Rearrange to:

$$V_1 / V_2 = [(f_2 / f_1) (L_2 / L_1) (D_1 / D_2)]^{.5}$$

This is one equation that relates v_1 and v_2 ; what is the other?



Hardy-Cross Method

- Q 's into a junction = Q 's out of a junction
- Head loss between any 2 junctions must be the same no matter what path is taken (head loss around a loop must be zero)



Steps

1. Choose a positive direction (CW=+)
2. # all pipes or identify all nodes
3. Divide network into independent loops such that each branch is included in at least one loop



4. Calculate K' for each pipe

- Calc. K' for each pipe

$$K' = (0.0252)fL/D^5$$

- For simplicity f is usually assumed to be the same (typical value is .02) in all parts of the network

5. Assume flow rates and directions



- Requires assumptions the first time around
- Must make sure that $Q_{in} = Q_{out}$ at each node

6. Calculate $Q_t - Q_a$ for each independent loop

- $Q_t - Q_a = -\sum K' Q_a^n / n \sum |Q_a^{n-1}|$
- $n=2$ (if Darcy-Weisbach is used)
- $Q_t - Q_a = -\sum K' Q_a^2 / 2 \sum |Q_a^{n-1}|$
- Q_t is true flow
- Q_a is assumed flow
- Once the difference is zero, the problem is completed



7. Apply $Q_t - Q_a$ to each pipe

- Use sign convention of step one
- $Q_t - Q_a$ (which can be + or -) is added to CW flows and subtracted from CCW flows
- If a pipe is common to two loops, two $Q_t - Q_a$ corrections are added to the pipe



8. Return to step 6

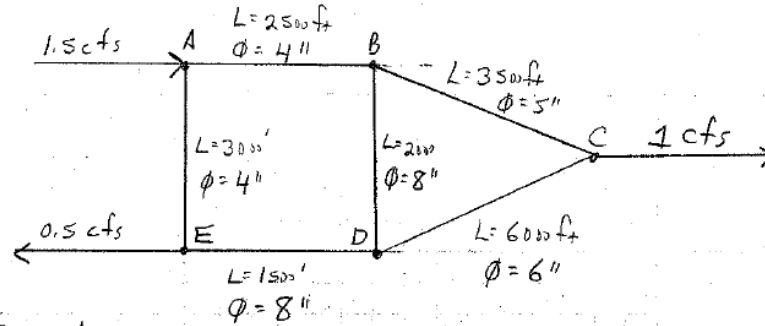
- Iterate until $Q_t - Q_a = 0$



Example Problem

- 2 loops; 6 pipes
 - By hand; 1 iteration
 - By spreadsheet

Calculate Q in each pipe of the network shown.



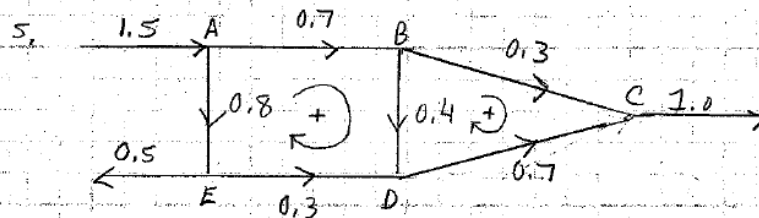
1. $CW = +$

2/3 2 loops
 Loop ABDE
 Loop BCD

4 $k' = \frac{(0.0252) f L}{D^5}$ f assumed to be 0.02 for all pipes

Pipe AB $k' = \frac{(0.0252)(0.02)(2500)}{(0.3333)^5} = 306.2$

- Pipe BC $k' = 140.5$
- DC $k' = 96.8$
- BD $k' = 7.7$
- ED $k' = 5.7$
- AE $k' = 367.4$



$$6. \quad \delta = \frac{-\sum K' Q_a^2}{2 \sum |K' Q_a|}$$

δ_{ABDE}

$$= \frac{-[(306.2)(0.7)^2 + (7.7)(0.4)^2 - (5.7)(0.3)^2 - (367.4)(0.8)^2]}{2 [(306.2)(0.7) + (7.7)(0.4) + (5.7)(0.3) + (367.4)(0.8)]}$$

$$\delta_{ABDE} = +0.08$$

δ_{BCD}

$$= \frac{-[(140.5)(0.3)^2 - (96.8)(0.7)^2 - (7.7)(0.4)^2]}{2 [(140.5)(0.3) + (96.8)(0.7) + (7.7)(0.4)]}$$

$$\delta = +0.16$$

7. Corrected flows

pipe AB	$0.7 + (0.08) = 0.78$
BC	$0.3 + (0.16) = 0.46$
DC	$0.7 - (0.16) = 0.54$
BD	$0.4 + (0.08) - (0.16) = 0.32$
ED	$0.3 - (0.08) = 0.22$
AE	$0.8 - (0.08) = 0.72$

Hardy-Cross		Solution to example problem																			
2-loops (6 pipes)				Lng (ft)		Dia. (ft)															
AB	K'=	306	AB	2500	0.33																
BD	K'=	7.7	BC	3500	0.42																
DE	K'=	5.7	DC	6000	0.50																
EA	K'=	368	BD	2000	0.67																
BC	K'=	140	ED	1500	0.67			f=		0.02											
CD	K'=	97	AE	3000	0.33																
DB	K'=	7.7																			
		Loop 1				Loop 2				Loop 1		Loop 2		Corrected Loop 1				Corrected Loop 2			
Iteration	Qa-b	Qb-d	Qd-e	Qe-a	Qb-c	Qc-d	Qdb	correction	correction	Qa-b	Qb-d	Qd-e	Qe-a	Qb-c	Qc-d	Qdb					
1	0.70	0.40	0.30	0.80	0.30	0.70	0.40	0.08	0.16	0.78	0.32	0.22	0.72	0.46	0.54	0.32					
2	0.78	0.32	0.22	0.72	0.46	0.54	0.32	0.00	-0.01	0.78	0.33	0.22	0.72	0.45	0.55	0.33					
3	0.78	0.33	0.22	0.72	0.45	0.55	0.33	0.00	0.00	0.78	0.33	0.22	0.72	0.45	0.55	0.33					
4	0.78	0.33	0.22	0.72	0.45	0.55	0.33	0.00	0.00	0.78	0.33	0.22	0.72	0.45	0.55	0.33					